

Received

DEC 9 2025

Town of East Lyme  
Land Use

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November 28, 2025

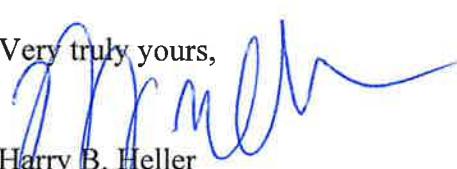
Town of East Lyme Zoning Commission  
Attn: Mr. William Mulholland, Zoning Enforcement Officer  
108 Pennsylvania Avenue  
Niantic, CT 06357

Re: Parkers Place, LLC

Dear Mr. Mulholland:

I am forwarding herewith a "Park Place Traffic Impact Assessment" prepared by William Kresic, P.E., PTOE and Priscilla Sawyer of VHB on behalf of Parkers Place, LLC in conjunction with the proposed multi-family residential development located on the northwesterly side of Park Place in East Lyme, Connecticut. Please add the traffic impact analysis to the record of this proceeding.

Should you have any questions, please feel free to contact the undersigned.

Very truly yours,  
  
Harry B. Heller

HBH/rmb  
enclosures

cc: Mr. Jason Pazzaglia  
Mr. Brandon Handfield

Received

DEC 9 2025

Town of East Lyme  
Land Use

To: Brandon Handfield  
Yantic River Consultants  
191 Norwich Avenue  
Lebanon, CT 06249

Date: October 24, 2025

Project #: 43273.00

From: William Kresic, P.E., PTOE  
Priscilla Sawyer

Re: Park Place Traffic Impact Assessment

VHB has been retained by Yantic River Consultants, LLC to conduct a traffic analysis for the proposed development of single family attached residential affordable housing located on Park Place in East Lyme, Connecticut. The proposed development can be seen on the attached site plan indicating 60 units. VHB has been retained to review the traffic generated by the site and any impacts the proposed development may have on the intersection of Park Place and West Main Street.

Under this project, the existing site will be developed into 60 two bedroom apartment units. Building and parking access will be provided through one full access site driveway located on Park Place.

## Trip Generation and Distribution

### Trip Generation

The vehicle trips the project is expected to generate were calculated based on trip generation rates provided in the ITE Trip Generation manual, 11th edition. The rate at which any development generates traffic is dependent upon a number of factors such as size, location, and concentration of surrounding developments. Land Use Codes (LUC) 223 for Affordable Housing was selected as the most conservative methodology for the development. The total net new trips generated by the proposed development would result in 34 (10 entering, 24 exiting) new trips in the AM peak hour and 36 (21 entering, 15 exiting) new trips in the PM peak hour. The trip generation associated with the proposed development program is shown in **Table 1**.

**Table 1**  
**Trip Generation Summary**  
**60 Affordable Housing Units**

Time Period	Apartment Trips <sup>1</sup>
<b>Weekday Morning</b>	
<b>Peak Hour<sup>2</sup></b>	
Enter	10
Exit	24
Total	34
<b>Weekday Evening</b>	
<b>Peak Hour<sup>2</sup></b>	
Enter	21
Exit	15
Total	36

<sup>1</sup> Based on ITE Land Use Code (LUC) 223 (Affordable Housing)  
Traffic volumes expressed in vehicles per hour

100 Great Meadow Road  
Suite 200  
Wethersfield, CT 06109  
P 860.807.4300

### **Trip Distribution**

Trip generation was calculated for the entire proposed facility. As the site is currently undeveloped, it was assumed that all trips are new trips impacting the study area roadways. The net addition of proposed site-generated trips is 34 (10 entering, 24 exiting) new trips in the AM peak hour and 36 (21 entering, 15 exiting) new trips in the evening peak hour. The ITE trip generation is included in the Appendix.

It was conservatively assumed that the transportation mode share for all trips to and from the site would be by automobile. The directional distribution of the vehicular traffic approaching and departing the site is a function of population densities, the location of employment, existing travel patterns, and the efficiency of the existing roadway system. Anticipated trip distribution patterns were based on the existing distribution of vehicle traffic on West Main Street, and Park Place from the traffic counts done by VHB on January 30, 2024.

The results of the evaluation are shown in **Table 2**, as well as **Figure 7**.

**Table 2      Trip Distribution**

Location	Traffic Entering/Leaving
West Main St East	50%
West Main St West	50%

The resulting site-generated trips will be split between the roadways as shown, directing traffic to downtown Niantic, East Lyme, and out of the town.

## Traffic Operations Analysis

To assess the quality of traffic operations within the study area, intersection capacity analyses were conducted for the 2025 existing conditions. In order to be conservative capacity analyses were conducted for the 2030 no-build conditions, and the 2030 build conditions. Capacity analyses provide an indication of the adequacy of the roadway facilities to serve the anticipated traffic demands.

### Method

The capacity analyses were conducted using industry standard Synchro software (Version 11).<sup>1</sup> To analyze unsignalized intersections, the HCM 6<sup>th</sup> Edition was used due to limitations analyzing all-way stop-controlled intersections in other previous versions.

Capacity analyses results are reported using a variety of performance measures, including "Level of Service" (LOS), Volume-to-Capacity (v/c) ratio, and queue length. These performance measures are described below.

#### Level-of-Service (LOS)

The level of service designation at intersections is based on the average control delay experienced by a vehicle traveling through the intersection. Similar to a report card, LOS designations are letter based, ranging from A to F, with LOS A representing the best operating condition (lowest vehicle delays) and LOS F representing the worst operating condition (highest vehicle delays). It is important to note that intersections during peak traffic conditions are not necessarily expected to operate at LOS A; an intersection operating at LOS A during typical peak conditions may suggest that the roadway is over-designed with too much capacity. Additionally, an intersection operating at LOS F during the peak periods does not necessarily indicate a failing intersection. In some cases, LOS F conditions may be deemed tolerable, provided that the lengthy delays do not lead to other, more serious conditions, such as increases in crash frequency.

LOS is assigned differently for signalized and unsignalized intersections. For signalized intersections, the analysis considers the operation of all traffic entering the intersection, and the LOS can be reported for individual turning movements, approaches, or for the intersection as a whole. For unsignalized intersections with stop-control on the side street approaches, the analysis assumes that through and right-turning movements on the main street are unimpeded by side street traffic. As such, LOS is reported only for left-turns from the main street and for all movements from the side street; the overall intersection LOS is not reported. Additionally, the delay values for each range are slightly longer for signalized intersections than unsignalized intersections. This is based on the presumption that the public will be more patient at signalized intersections where they are guaranteed entry into the intersection in a reasonable amount of time.

#### Volume-to-Capacity (v/c)

Volume-to-Capacity (v/c) can be reported for individual lane groups or for the intersection as a whole. Volume-to-Capacity is a ratio comparing the volume of vehicles proceeding through an intersection (or lane group) with the capacity of the intersection (or lane group) to accommodate that volume. A v/c ratio of 1.0 indicates an intersection operating at its capacity.

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<sup>1</sup> *Highway Capacity Manual*, Transportation Research Board, Washington, DC (2000).

### **Queue Length**

Vehicle queues are the expected length of vehicles waiting at an intersection. This report documents the 50<sup>th</sup> percentile and 95<sup>th</sup> percentile vehicle queue lengths in feet. The 50<sup>th</sup> percentile queue represents the maximum vehicle queue length that is expected to occur during a signal cycle under typical (median) traffic flow rates during the peak period analyzed. The 95<sup>th</sup> percentile queue represents the maximum vehicle queue length that is expected to be exceeded only during the 5-percent of signal cycles (two or three times per hour) in the peak hour with the heaviest traffic flow. The 95<sup>th</sup> percentile queue is generally considered the maximum vehicle queue for design purposes.

### **Unsignalized Intersection Capacity Analysis**

**Table 3** presents a summary of the capacity analyses for the unsignalized study intersection. The Synchro analysis reports are included in the Appendix.

The results indicate that Park Place at the Site Driveway will attain acceptable LOS through 2030 build conditions.

**Park Place at Site Driveway:** Under Build conditions the intersection is expected to operate with LOS A due to the very minimal vehicle volumes leaving the site. Queuing and delays are expected to be minimal during the weekday evening peak hour and morning peak hour with the development of the site.

**Park Place at West Main St:** Park Place at West Main Street operates with LOS C or better under all peak hours during existing conditions and is expected to do so under future No-Build conditions as well. Under Build conditions the level of service is expected to remain LOS C or better with the new projected traffic. Additional site traffic is not anticipated to significantly impact queuing and delays are expected to be remain similar to existing conditions during the weekday evening peak hour and morning peak hour.

**Table 3 Unsignalized Intersection Capacity Analysis Summary**

Location	Peak Hour	Mov't	2025 Existing Conditions					2030 No-Build Conditions					2030 Build Conditions				
			Dem. <sup>1</sup>	v/c <sup>2</sup>	Del <sup>3</sup>	LOS <sup>4</sup>	Q95 <sup>5</sup>	Dem.	v/c	Del	LOS	Q95	Dem.	v/c	Del	LOS	Q95
W Main Street at Park Place	AM	EB L/T	244	0.003	7.8	A	0	254	0.003	7.8	A	0	259	0.007	7.9	A	0
		WB T/R	218	0	0	A	0	226	0	0	A	0	231	0	0	A	0
		SB L/R	17	0.043	11.7	B	0.1	17	0.044	11.9	B	0.1	41	0.103	12.2	B	0.3
	PM	EB L/T	330	0.011	8.3	A	0	343	0.011	8.4	A	0	353	0.021	8.4	A	0.1
		WB T/R	398	0	0	A	0	415	0	0	A	0	426	0	0	A	0
		SB L/R	14	0.064	15.2	C	0.2	14	0.067	15.7	C	0.2	29	0.134	16.1	C	0.5
Park Place at Site Driveway	AM	EB L/R											24	0.025	8.5	A	0.1
		NB L/T											22	0.007	7.3	A	0
		SB T/R											17	0	0	A	0
	PM	EB L/R											15	0.015	8.4	A	0
		NB L/T											48	0.014	7.3	A	0
		SB T/R											14	0	0	A	0

Source: VHB, Inc. using Synchro 11 software and HCM 6<sup>th</sup> TWSC Edition methodology.

1 demand, in vehicles per hour

2 volume-to-capacity ratio for the critical movement

3 delay of critical approach only, in seconds

4 level of service

5 95<sup>th</sup> percentile queue length, in vehicles

EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound; R = right; T = through, L = left

100 Great Meadow Road

Suite 200

Wethersfield, CT 06109

P 860.807.4300

## Crash History

To identify potential vehicle crash trends and/or roadway deficiencies within the project study area, VHB conducted a review of the Connecticut Crash Data Repository (CTCDR) developed by UConn for the most recent three years of crash data collection (January 2021 – December 2024). To be reported, \$1000 of property damage or an injury must have occurred.

There were no crashes reported near the study intersection of West Main Street and Park Place or in front of the proposed site on Park Place.

## Intersection Sight Distance

The proposed site will provide vehicular access via Park Place and West Main Street as shown on the site plan.

The posted speed limit on Park Place is 25 mph. Using an estimated 85<sup>th</sup> percentile speed of 30 mph, Connecticut Department of Transportation requires 280 feet of Intersection Site Distance. Intersection sight distances were observed at the location of the proposed site driveway. Based on the results of this investigation, adequate intersection sight distance is available for the right of the site driveway. In order to meet requirements, to the left of the site driveway some tree trimming will be needed.

The posted speed limit on West Main Street is 25 mph. Using a conservative estimated 85<sup>th</sup> percentile speed of 35 mph, Connecticut Department of Transportation requires 350 feet of Intersection Site Distance. Intersection sight distances were observed at the intersection for Park Place. The measured intersection site distances meet requirements.

Results of the sight distance investigation are summarized in **Table 4**.

**Table 4      Intersection Sight Distances**

Location	Available Sight Distance			Meets Standard	
	Left	Right	Minimum	Left	Right
Park Pl at W Main St	>350'	>350'	350'	Yes	Yes
Park Pl at Site Drive	250'	N/A	280'	No	Yes

Source: Vanasse Hangen Brustlin, Inc.

## Site Access and Parking

This Site is proposed to have access via a full access driveway on Park Place.

The proposed full access site driveway is proposed to be located approximately 325 feet north of the intersection of Park Place at West Main Street. This driveway is to operate under stop control and allow for access to all uses on site.

Based on the current site plan, a total of 146 parking spaces are to be provided on site.

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Suite 200  
Wethersfield, CT 06109  
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## Conclusion

This document has outlined the anticipated transportation impacts of the proposed development of 60 single family attached residential affordable housing units on Park Place in East Lyme, Connecticut.

The traffic generated by the proposed site is not anticipated to adversely impact operations on Park Place or West Main Street, the LOS is anticipated to remain at a level of C or better and the queuing and the delay time is expected to remain acceptable. The crash history does not indicate any issues at the intersections and the available sight distances at the site driveway should be sufficient given the tree trimming mentioned in the intersection sight distance portion of this report. The addition of the Park Place Multi-Family Development is anticipated to generate 34 (10 entering, 24 exiting) new trips in the AM peak hour and 36 (21 entering, 15 exiting). Given the limited traffic generated by the site and the traffic distribution as shown, the site traffic is anticipated to have no significant impact to traffic operations in the area of the proposed site driveway.

# Appendix

Figures

Site Plans

Traffic Counts

Project Trip Generation

Capacity Analysis Reports

## Figures

Figure 1

Park Place Traffic Impact Assessment | East Lyme, CT

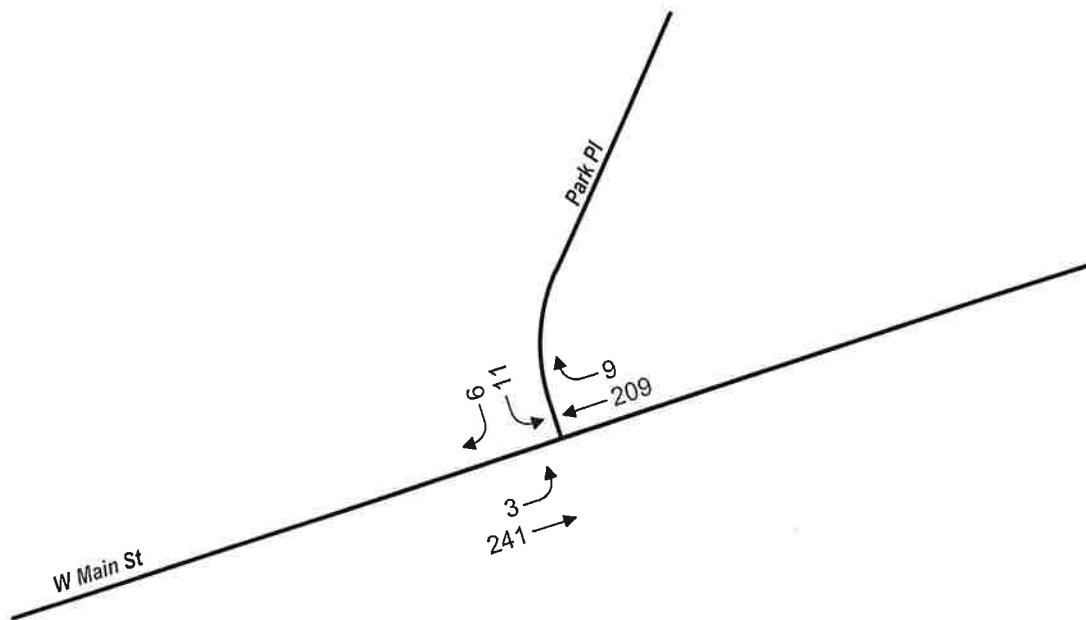


Existing AM

| Oct 22, 2025

Ⓐ Signalized Intersection

neg = Negligible



Not to Scale

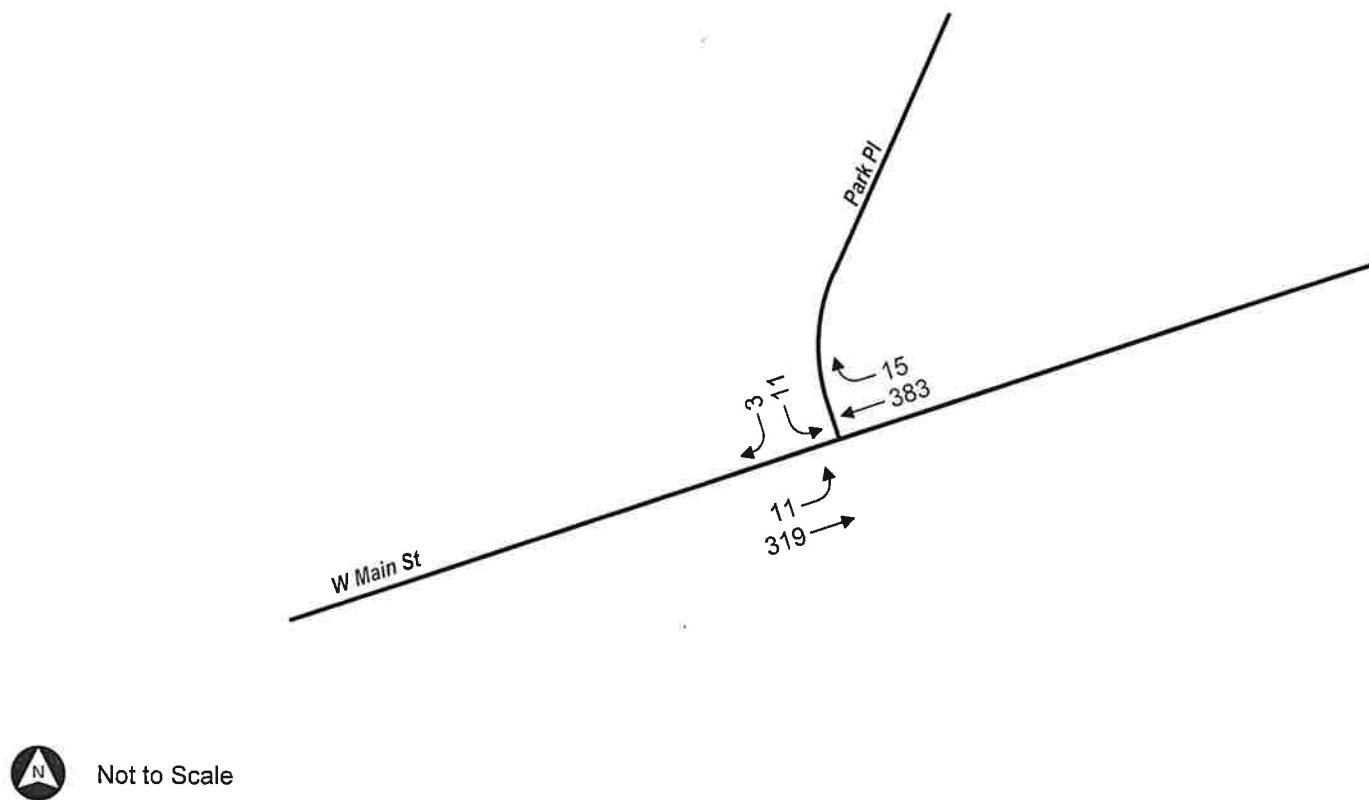
**Figure 2**

Park Place Traffic Impact Assessment | East Lyme, CT



Existing PM | Oct 22, 2025

(S) **Signalized Intersection**  
neg = **Negligible**



**Figure 3**

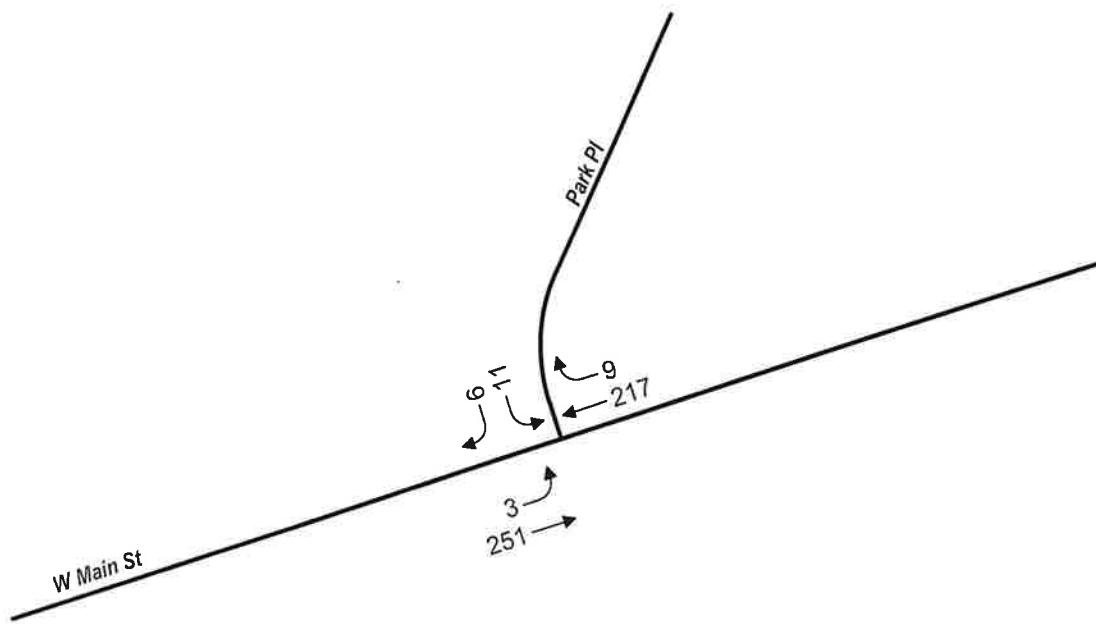
Park Place Traffic Impact Assessment | East Lyme, CT



No Build AM | Oct 22, 2025

Ⓐ **Signalized Intersection**

neg = **Negligible**



Not to Scale

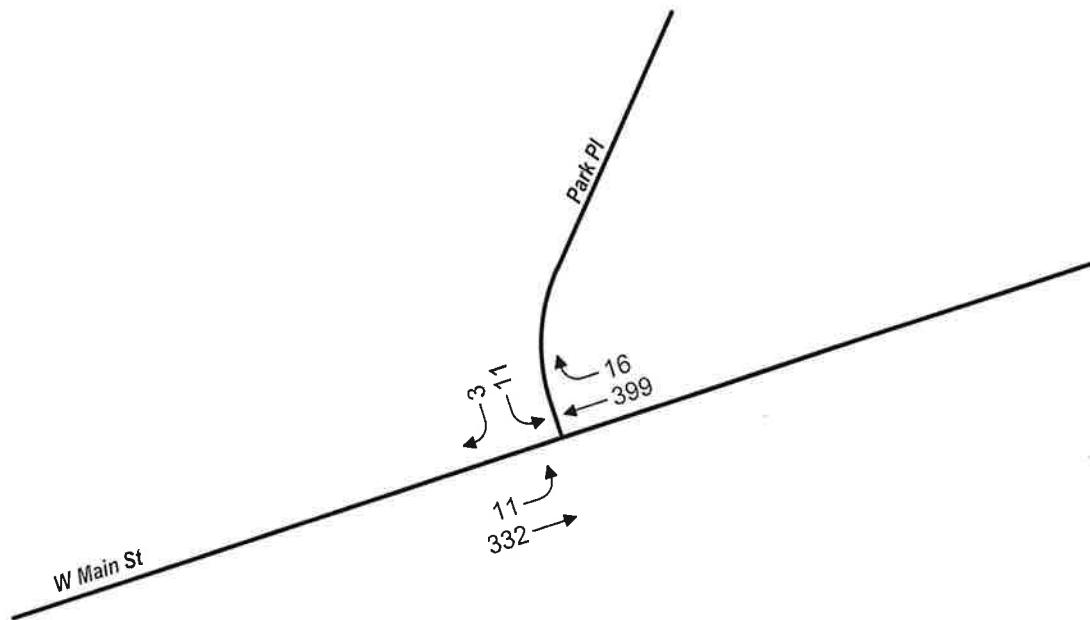
**Figure 4**

Park Place Traffic Impact Assessment | East Lyme, CT



No Build PM | Oct 22, 2025

⑤ **Signalized Intersection**  
neg = **Negligible**



Not to Scale

Figure 5

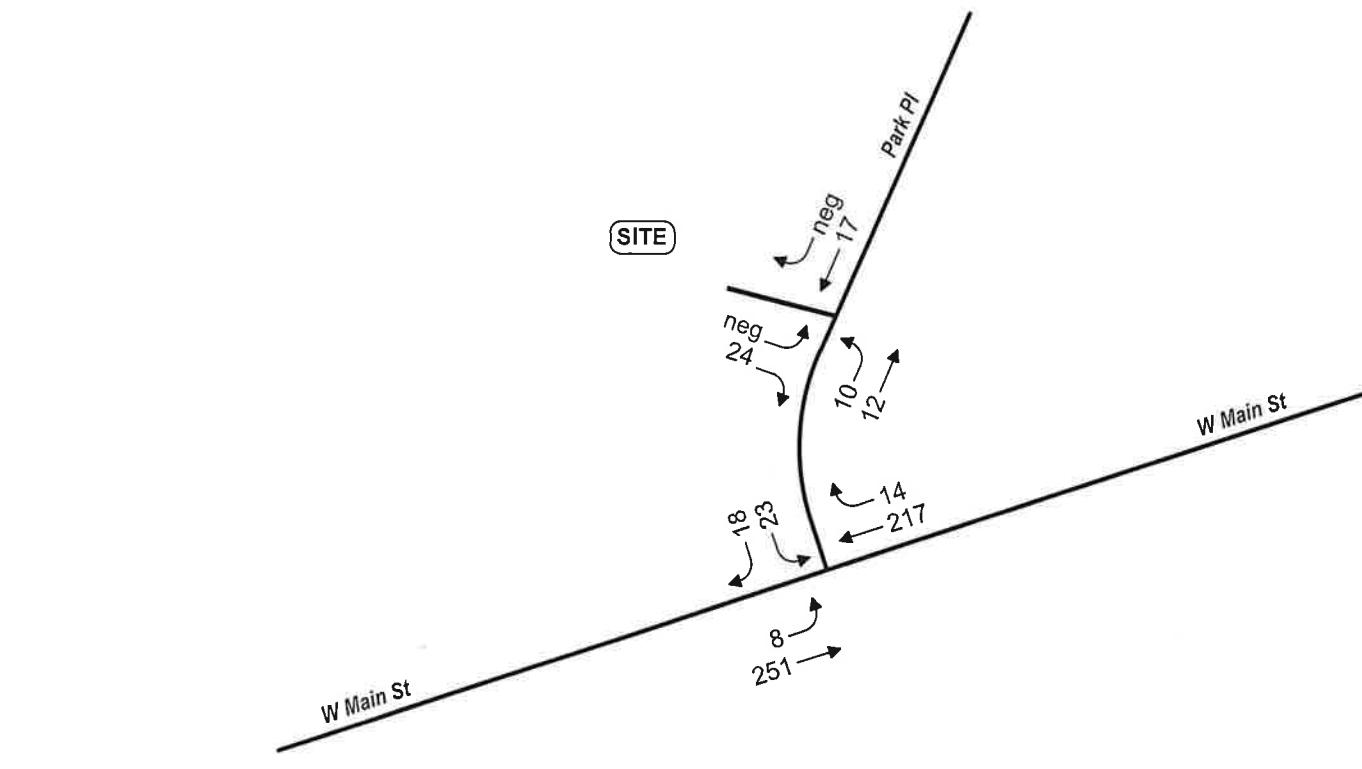
Park Place Traffic Impact Assessment | East Lyme, CT



Build AM | Oct 22, 2025

Ⓐ Signalized Intersection

neg = Negligible



Not to Scale

Figure 6

Park Place Traffic Impact Assessment | East Lyme, CT

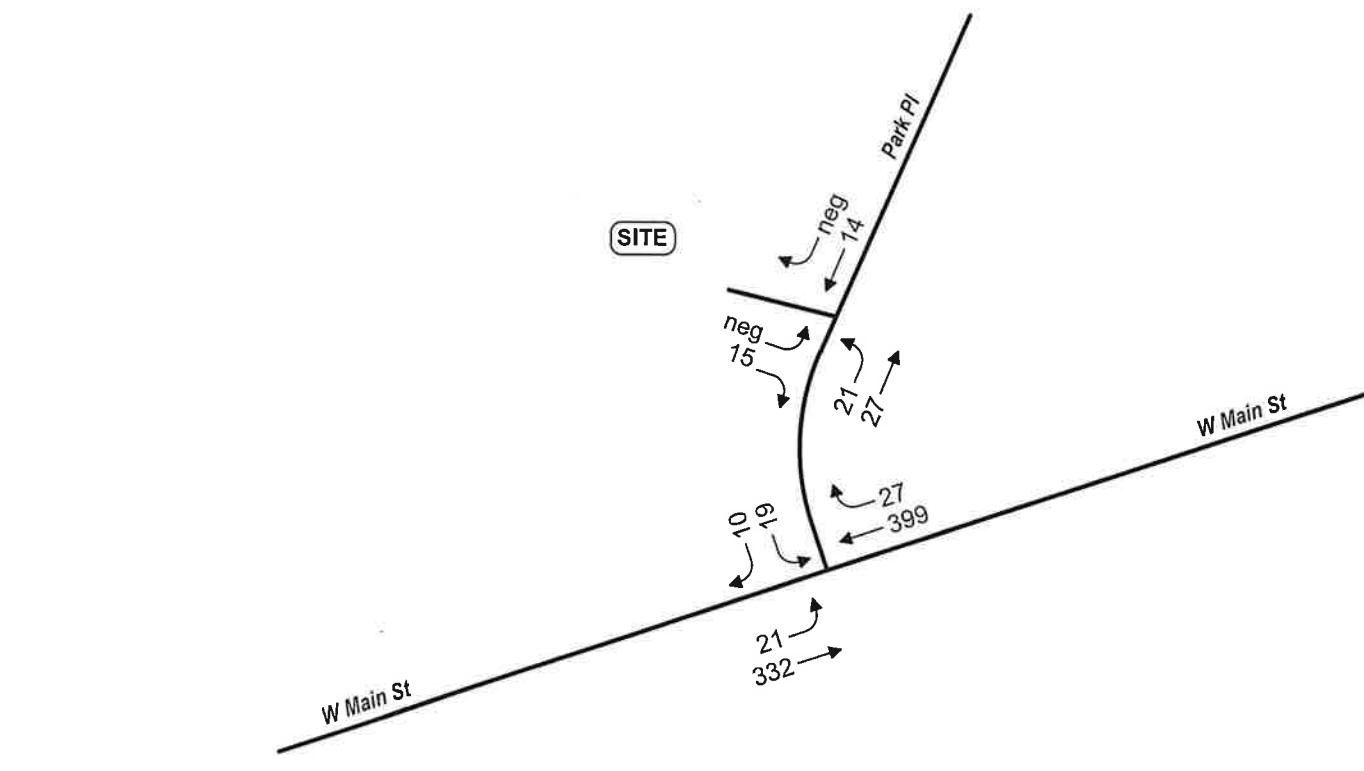


Build PM

| Oct 22, 2025

Ⓐ Signalized Intersection

neg = Negligible



Not to Scale

Figure 7

Park Place Traffic Impact Assessment | East Lyme, CT



Trip Distribution

| Oct 22, 2025

Ⓐ Signalized Intersection

neg = Negligible

xx = Entering Trips

(xx) = Exiting Trips

➡ Regional Trip Distribution

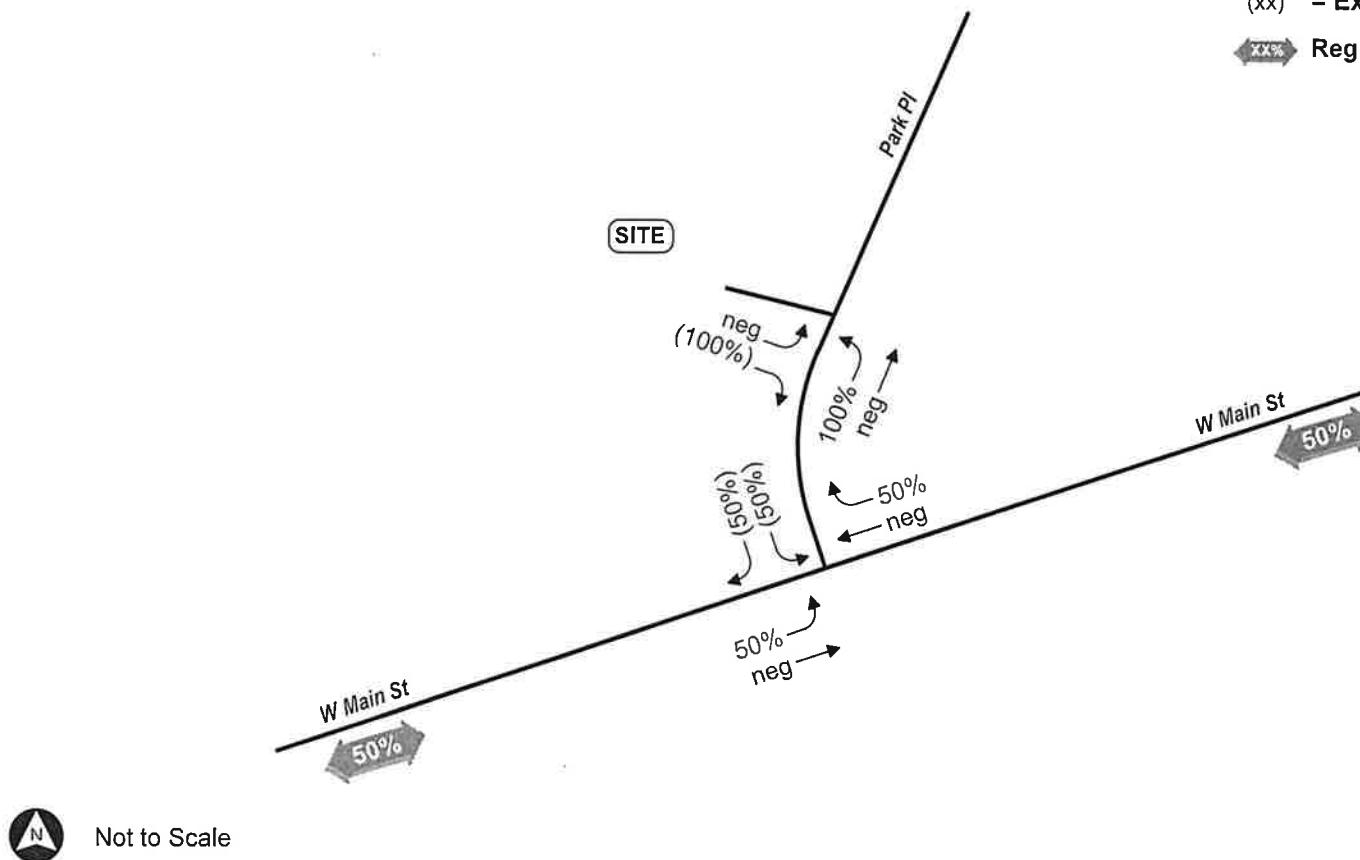


Figure 8

Park Place Multi-Family Development | East Lyme, CT



Trip Generation AM

| Oct 22, 2025

(S) **Signalized Intersection**

neg = **Negligible**

xx = **Entering Trips**

(xx) = **Exiting Trips**

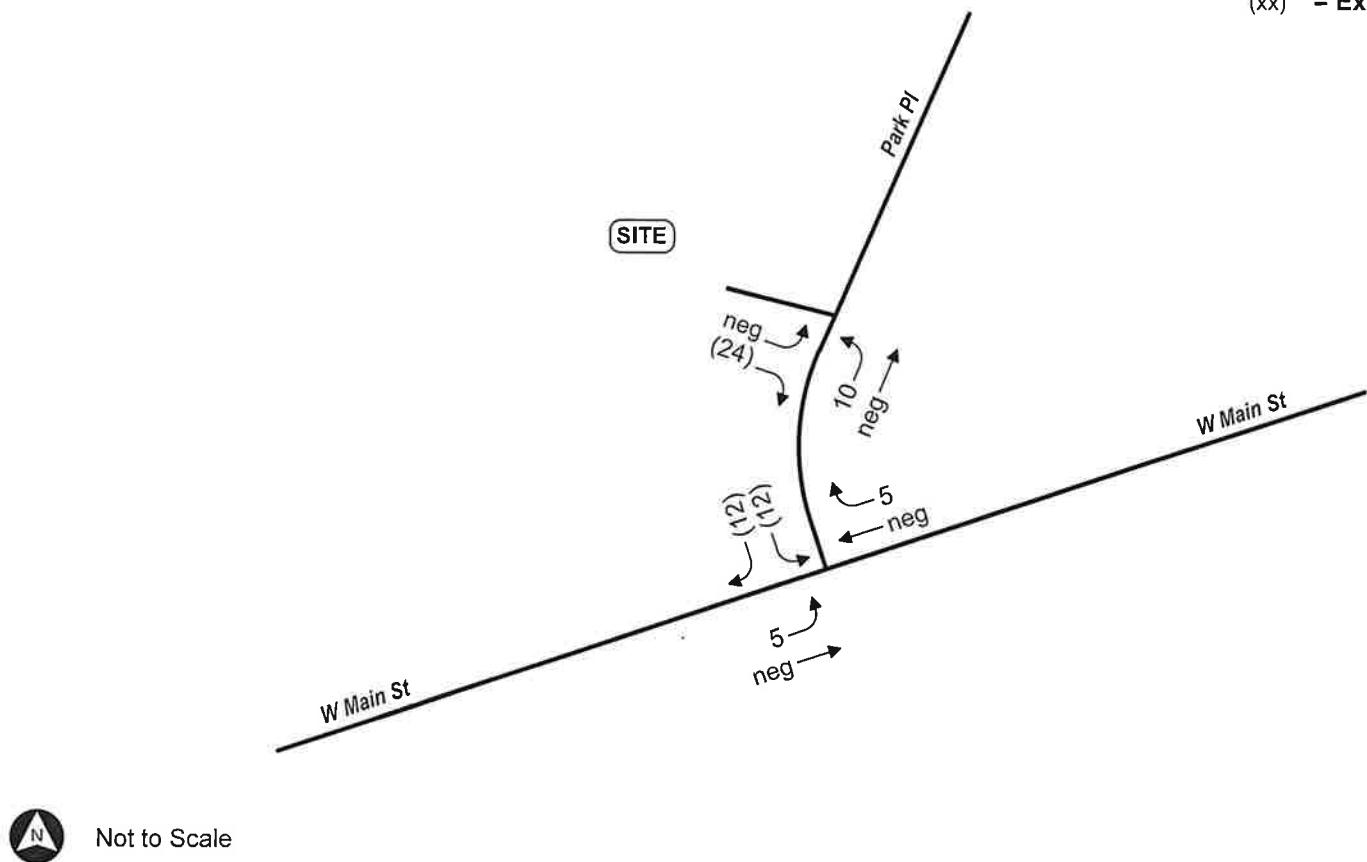


Figure 9

Park Place Multi-Family Development | East Lyme, CT



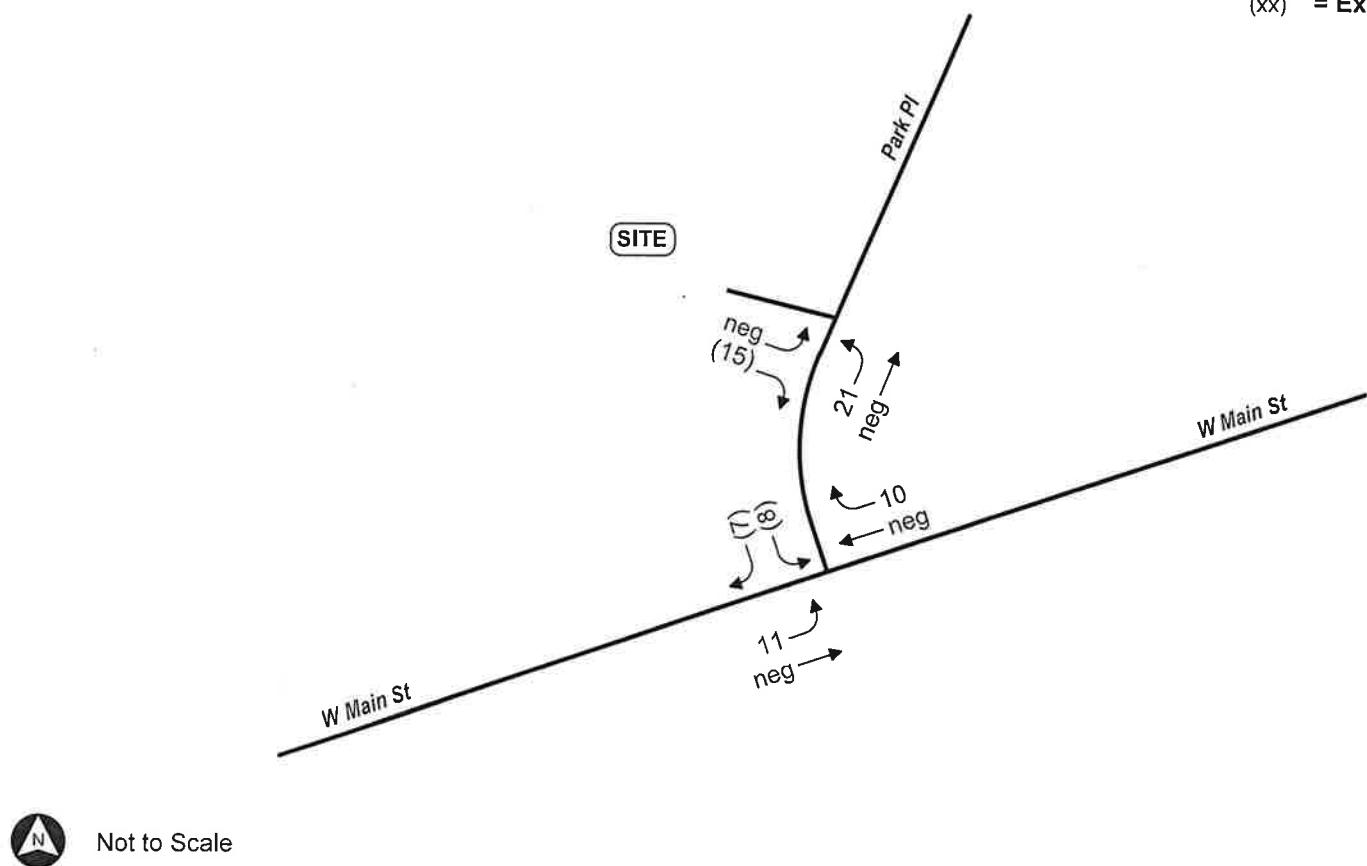
Trip Generation PM | Oct 22, 2025

(S) **Signalized Intersection**

neg = **Negligible**

xx = **Entering Trips**

(xx) = **Exiting Trips**

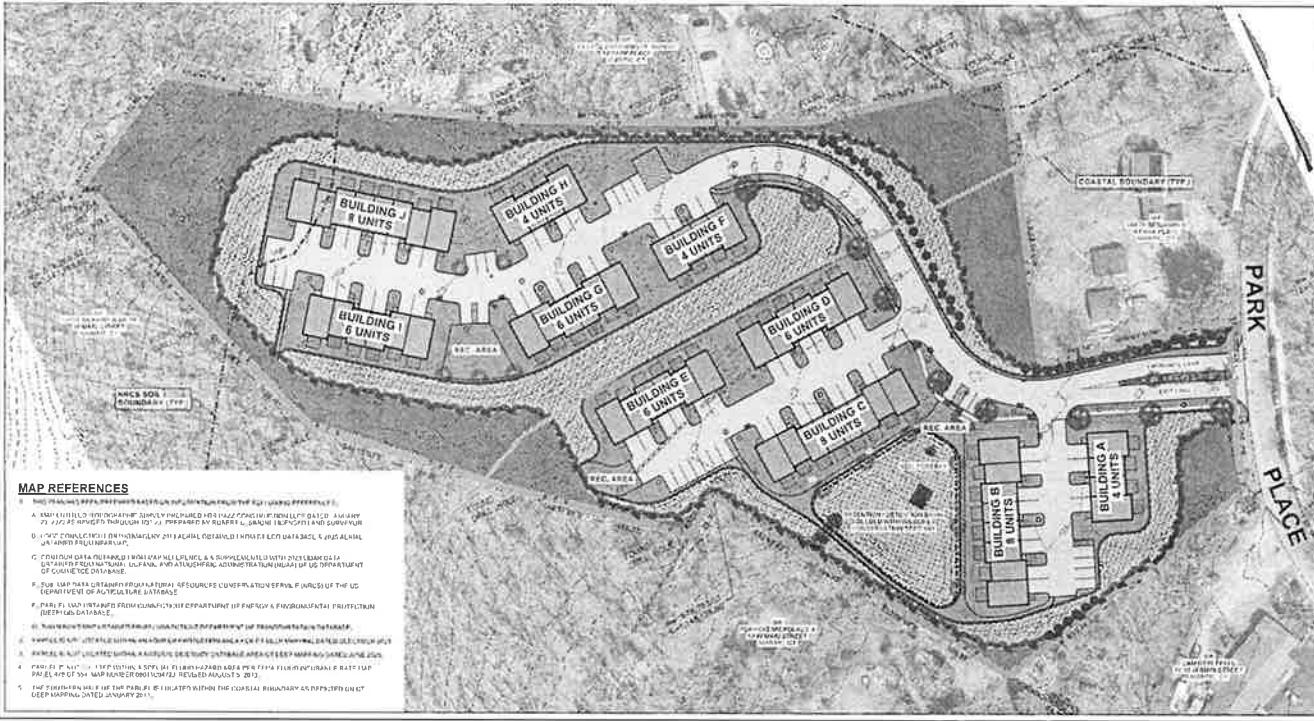


## Site Plans

**PARKERS PLACE  
MULTI-FAMILY DEVELOPMENT  
PARK PLACE (MAP 11.1, LOT 11)  
EAST LYME, CONNECTICUT**

**PREPARED FOR**

OWNER / APPLICANT  
PARKERS PLACE LLC  
PO BOX 817  
EAST LYME, CT 06333



1" = 60'



**CONTACT INFORMATION**  
YANTIC HONEY CONSULTANTS, LLC  
191 NORWICH AVENUE  
LEBANON, CONNECTICUT 06249  
Phone: (860) 467-7268  
Email: [YanticHoney@gmail.com](mailto:YanticHoney@gmail.com)  
Web: [www.yantic-honey.com](http://www.yantic-honey.com)



11

## INDEX OF SHEETS

DESCRIPTION	SHEET NUMBER
TITLE SHEET	1
DEVELOPMENT LAYOUT PLAN	2
GRADING & DRAINAGE PLAN	3
UTILITY PLAN	4
GENERAL NOTES	5

SCALE: 1" = 60'

50	0	50	100	150
<b>PROJECT NUMBER:</b> _____				

1000

YANTIC RIVER CONSULTANTS, LLC  
191 NORWICH AVENUE  
LEBANON, CONNECTICUT 06249

11

BARKERS PLACE

**PARKERS PLACE  
MULTI-FAMILY DEVELOPMENT**

PREPARED FOR  
ERS PLACE LLC, OWNER / APPLICANT

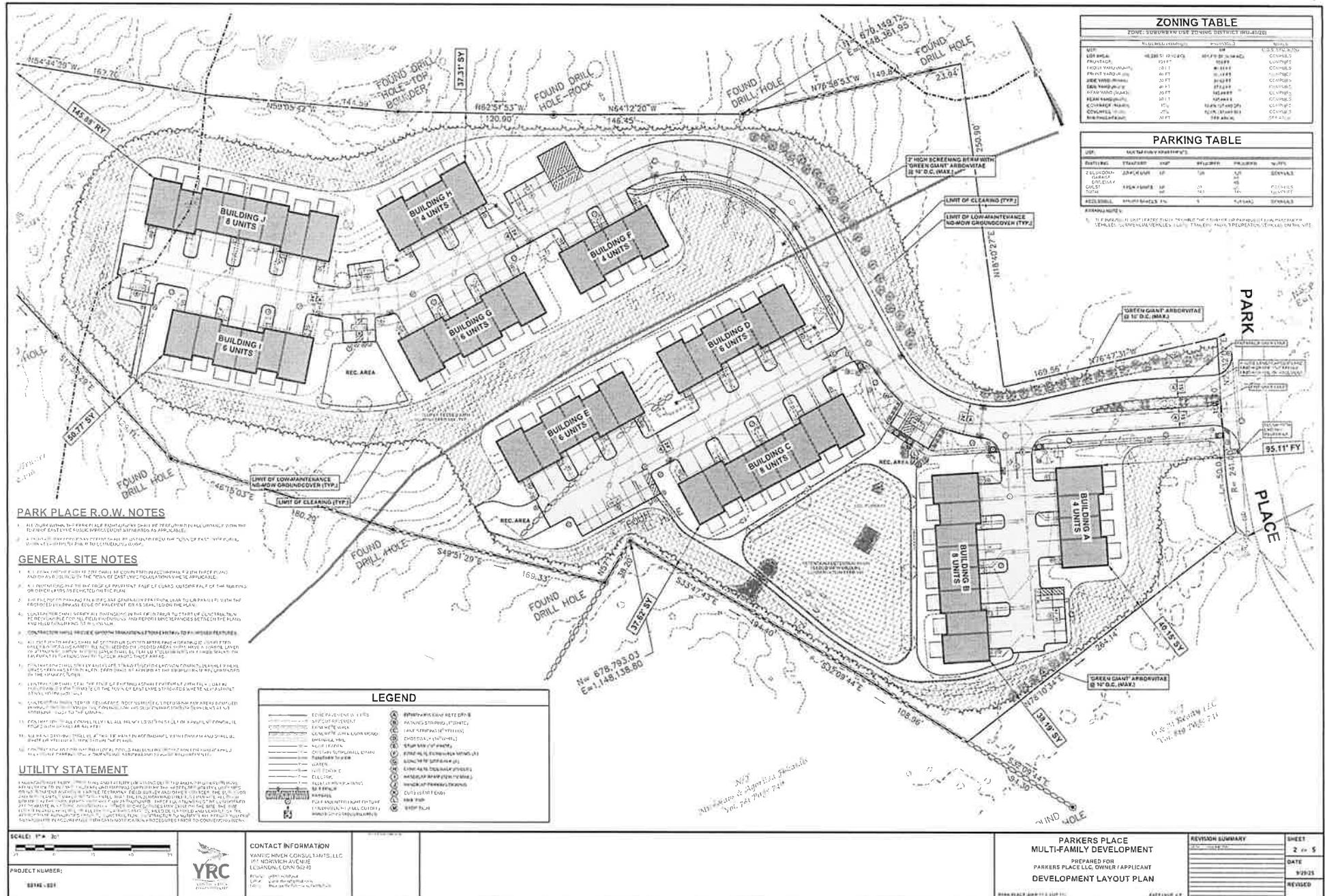
## TITLE SHEET

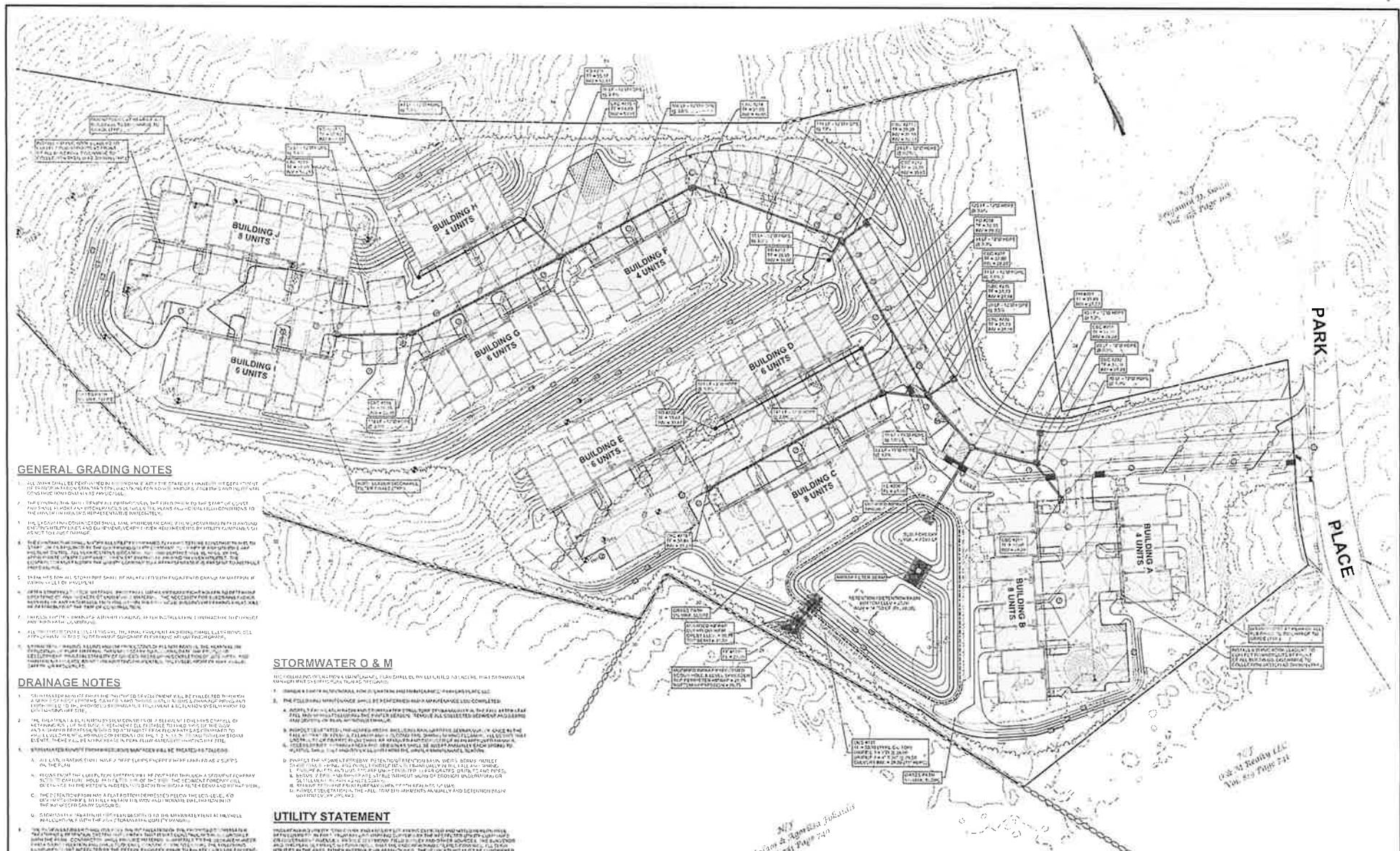
REVIEWED BY

### ANSWER

1

10





Macmillan 247  
Vol. 747 Page 74n  
McGraw-Hill Book Company  
1957

LEVEL 1 • 3



**CONTACT INFORMATION**  
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— 3 —

1000

23 0

ACT NUMBER

20145-221

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**PARKERS PLACE  
MULTI-FAMILY DEVELOPMENT**

**PREPARED FOR  
PARKERS PLACE LLC, OWNER / APPLICANT**

**GRADING & DRAINAGE PLAN**

REVISION SUMMARY	SHEET
	3 of 5
	DATE
	10/13/23
	REVISED





## Traffic Counts

**National Data & Surveying Services**  
**Intersection Turning Movement Count**

**Location:** Park Pl & W Main St  
**City:** East Lyme  
**Control:** 1-Way Stop(SB)

**Project ID:** 24-410007-001  
**Date:** 1/30/2024

**Data - Total**

NS/EW Streets:	Park Pl				Park Pl				W Main St				W Main St								
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND								
AM	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	TOTAL	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	WT	WR	WU	TOTAL	
7:00 AM	0	0	0	0	2	0	0	0	0	44	0	0	0	50	0	0	0	0	0	96	
7:15 AM	0	0	0	0	3	0	2	0	0	35	0	0	0	54	2	0	0	0	0	96	
7:30 AM	0	0	0	0	7	0	1	0	0	57	0	0	0	64	0	0	0	0	0	129	
7:45 AM	0	0	0	0	2	0	1	0	0	71	0	0	0	55	1	0	0	0	0	130	
8:00 AM	0	0	0	0	3	0	0	0	1	62	0	0	0	40	3	0	0	0	0	109	
8:15 AM	0	0	0	0	2	0	4	0	0	46	0	0	0	41	1	0	0	0	0	94	
8:30 AM	0	0	0	0	1	0	2	0	1	64	0	0	0	58	1	0	0	0	0	127	
8:45 AM	0	0	0	0	5	0	0	0	1	67	0	0	0	68	1	0	0	0	0	142	
<b>TOTAL VOLUMES :</b>	<b>NL</b>	<b>NT</b>	<b>NR</b>	<b>NU</b>	<b>SL</b>	<b>ST</b>	<b>SR</b>	<b>SU</b>	<b>EL</b>	<b>ET</b>	<b>ER</b>	<b>EU</b>	<b>WL</b>	<b>WT</b>	<b>WR</b>	<b>WU</b>	<b>WT</b>	<b>WR</b>	<b>WU</b>	<b>TOTAL</b>	
<b>APPROACH %'s :</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>25</b>	<b>0</b>	<b>10</b>	<b>0</b>	<b>3</b>	<b>446</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>430</b>	<b>9</b>	<b>0</b>	<b>0</b>	<b>97.95%</b>	<b>2.05%</b>	<b>0.00%</b>	<b>923</b>
<b>PEAK HR :</b>	<b>08:00 AM - 09:00 AM</b>																				<b>TOTAL</b>
<b>PEAK HR VOL :</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>11</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>3</b>	<b>239</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>207</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0.00%</b>	<b>0.00%</b>	<b>0.00%</b>	<b>472</b>
<b>PEAK HR FACTOR :</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.550</b>	<b>0.000</b>	<b>0.375</b>	<b>0.000</b>	<b>0.750</b>	<b>0.892</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.761</b>	<b>0.500</b>	<b>0.000</b>	<b>0.772</b>	<b>0.831</b>			
<b>PM</b>	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND								
	0	0	0	0	0	1	0	0	0	1	0	0	0	1	0	0	0	0	0	0	TOTAL
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	WT	WR	WU	TOTAL	
4:00 PM	0	0	0	0	2	0	0	0	5	80	0	0	0	94	3	0	0	0	0	184	
4:15 PM	0	0	0	0	4	0	2	0	3	83	0	0	0	110	5	0	0	0	0	207	
4:30 PM	0	0	0	0	3	0	0	0	1	80	0	0	0	98	1	0	0	0	0	183	
4:45 PM	0	0	0	0	2	0	1	0	2	73	0	0	0	78	6	0	0	0	0	162	
5:00 PM	0	0	0	0	3	0	0	0	2	83	0	0	0	90	2	0	0	0	0	180	
5:15 PM	0	0	0	0	1	0	3	0	1	70	0	0	0	80	1	0	0	0	0	156	
5:30 PM	0	0	0	0	0	0	1	0	1	63	0	0	0	59	1	0	0	0	0	125	
5:45 PM	0	0	0	0	2	0	1	0	0	38	0	0	0	68	2	0	0	0	0	111	
<b>TOTAL VOLUMES :</b>	<b>NL</b>	<b>NT</b>	<b>NR</b>	<b>NU</b>	<b>SL</b>	<b>ST</b>	<b>SR</b>	<b>SU</b>	<b>EL</b>	<b>ET</b>	<b>ER</b>	<b>EU</b>	<b>WL</b>	<b>WT</b>	<b>WR</b>	<b>WU</b>	<b>WT</b>	<b>WR</b>	<b>WU</b>	<b>TOTAL</b>	
<b>APPROACH %'s :</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>17</b>	<b>0</b>	<b>8</b>	<b>0</b>	<b>15</b>	<b>570</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>677</b>	<b>21</b>	<b>0</b>	<b>0</b>	<b>96.99%</b>	<b>3.01%</b>	<b>0.00%</b>	<b>1308</b>
<b>PEAK HR :</b>	<b>04:00 PM - 05:00 PM</b>																				<b>TOTAL</b>
<b>PEAK HR VOL :</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>11</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>11</b>	<b>316</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>380</b>	<b>15</b>	<b>0</b>	<b>0</b>	<b>0.00%</b>	<b>0.00%</b>	<b>0.00%</b>	<b>736</b>
<b>PEAK HR FACTOR :</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.688</b>	<b>0.000</b>	<b>0.375</b>	<b>0.000</b>	<b>0.550</b>	<b>0.952</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.864</b>	<b>0.625</b>	<b>0.000</b>	<b>0.859</b>	<b>0.889</b>			

## Project Trip Generation

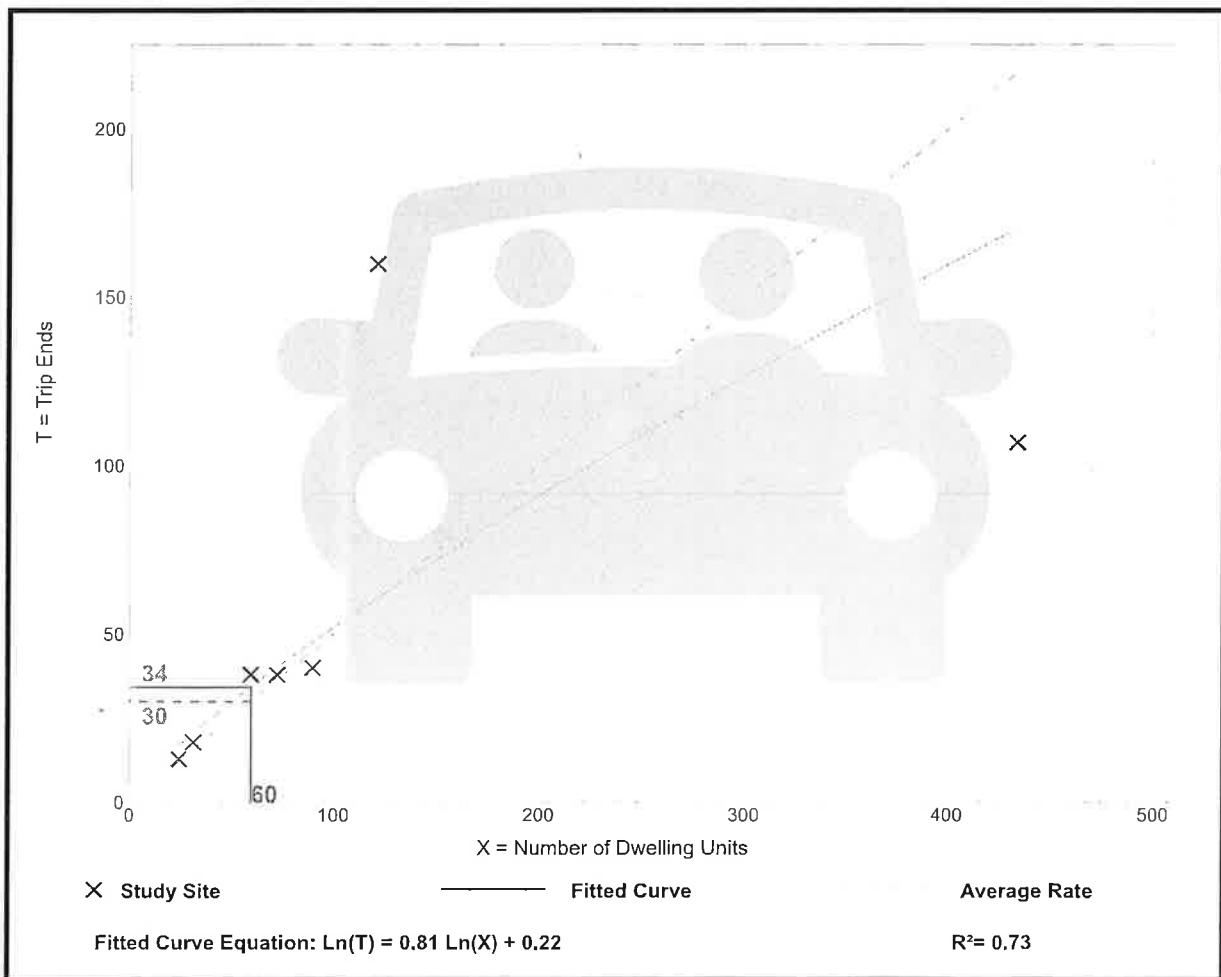
## Affordable Housing - Income Limits (223)

Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 7 and 9 a.m.  
Setting/Location: General Urban/Suburban  
Number of Studies: 7  
Avg. Num. of Dwelling Units: 119  
Directional Distribution: 29% entering, 71% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.50	0.25 - 1.32	0.39

### Data Plot and Equation



## Affordable Housing - Income Limits (223)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 8

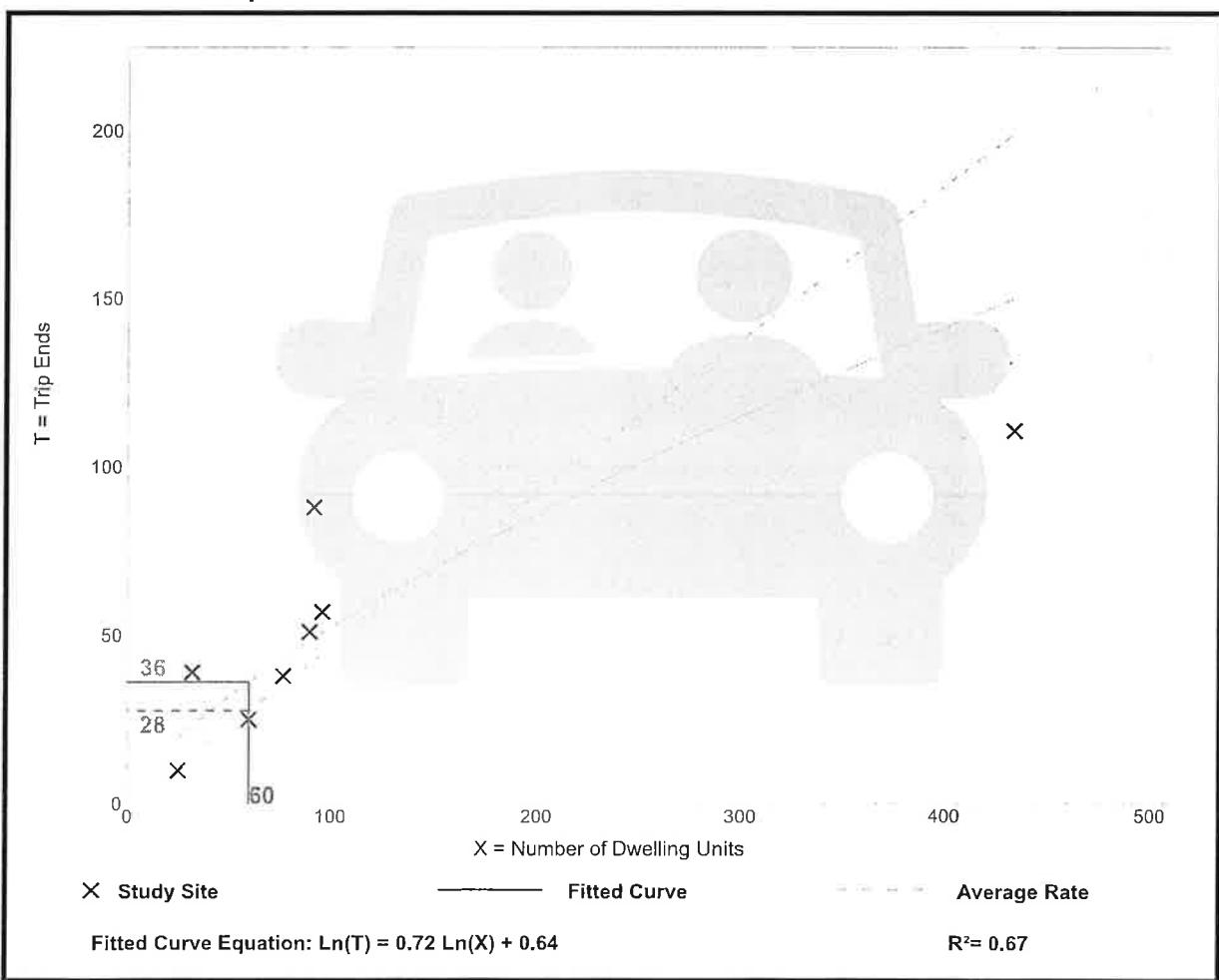
Avg. Num. of Dwelling Units: 113

Directional Distribution: 59% entering, 41% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.46	0.26 - 1.22	0.28

### Data Plot and Equation



## Capacity Analysis Reports

Intersection						
Int Delay, s/veh	1.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		Y	
Traffic Vol, veh/h	8	251	217	14	23	18
Future Vol, veh/h	8	251	217	14	23	18
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	89	89	77	77	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	282	282	18	32	25

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	300	0	-	0	591	291
Stage 1	-	-	-	-	291	-
Stage 2	-	-	-	-	300	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1261	-	-	-	470	748
Stage 1	-	-	-	-	759	-
Stage 2	-	-	-	-	752	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1261	-	-	-	466	748
Mov Cap-2 Maneuver	-	-	-	-	466	-
Stage 1	-	-	-	-	753	-
Stage 2	-	-	-	-	752	-

Approach	EB	WB	SB			
HCM Control Delay, s	0.2	0	12.2			
HCM LOS			B			

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1261	-	-	-	558	
HCM Lane V/C Ratio	0.007	-	-	-	0.103	
HCM Control Delay (s)	7.9	0	-	-	12.2	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.3	

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	↖		↑	↑		
Traffic Vol, veh/h	0	24	10	12	17	0
Future Vol, veh/h	0	24	10	12	17	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	26	11	13	18	0
Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	53	18	18	0	-	0
Stage 1	18	-	-	-	-	-
Stage 2	35	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	955	1061	1599	-	-	-
Stage 1	1005	-	-	-	-	-
Stage 2	987	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	948	1061	1599	-	-	-
Mov Cap-2 Maneuver	948	-	-	-	-	-
Stage 1	998	-	-	-	-	-
Stage 2	987	-	-	-	-	-
Approach	EB	NB	SB			
HCM Control Delay, s	8.5	3.3	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1599	-	1061	-	-	
HCM Lane V/C Ratio	0.007	-	0.025	-	-	
HCM Control Delay (s)	7.3	0	8.5	-	-	
HCM Lane LOS	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	0.1	-	-	

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		Y	
Traffic Vol, veh/h	21	332	399	27	19	10
Future Vol, veh/h	21	332	399	27	19	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	86	86	58	58
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	22	349	464	31	33	17
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	495	0	-	0	873	480
Stage 1	-	-	-	-	480	-
Stage 2	-	-	-	-	393	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1069	-	-	-	321	586
Stage 1	-	-	-	-	622	-
Stage 2	-	-	-	-	682	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1069	-	-	-	313	586
Mov Cap-2 Maneuver	-	-	-	-	313	-
Stage 1	-	-	-	-	606	-
Stage 2	-	-	-	-	682	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.5	0	16.1			
HCM LOS			C			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1069	-	-	-	373	
HCM Lane V/C Ratio	0.021	-	-	-	0.134	
HCM Control Delay (s)	8.4	0	-	-	16.1	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5	

Intersection						
Int Delay, s/veh	3.6					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			Y	Y	
Traffic Vol, veh/h	0	15	21	27	14	0
Future Vol, veh/h	0	15	21	27	14	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	16	23	29	15	0

Major/Minor	Minor2	Major1	Major2			
Conflicting Flow All	90	15	15	0	-	0
Stage 1	15	-	-	-	-	-
Stage 2	75	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	910	1065	1603	-	-	-
Stage 1	1008	-	-	-	-	-
Stage 2	948	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	896	1065	1603	-	-	-
Mov Cap-2 Maneuver	896	-	-	-	-	-
Stage 1	993	-	-	-	-	-
Stage 2	948	-	-	-	-	-

Approach	EB	NB	SB			
HCM Control Delay, s	8.4	3.2	0			
HCM LOS	A					

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR	
Capacity (veh/h)	1603	-	1065	-	-	
HCM Lane V/C Ratio	0.014	-	0.015	-	-	
HCM Control Delay (s)	7.3	0	8.4	-	-	
HCM Lane LOS	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	0	-	-	

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		Y	
Traffic Vol, veh/h	3	241	209	9	11	6
Future Vol, veh/h	3	241	209	9	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	89	89	77	77	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	271	271	12	15	8

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	283	0	-	0	554	277
Stage 1	-	-	-	-	277	-
Stage 2	-	-	-	-	277	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1279	-	-	-	493	762
Stage 1	-	-	-	-	770	-
Stage 2	-	-	-	-	770	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1279	-	-	-	492	762
Mov Cap-2 Maneuver	-	-	-	-	492	-
Stage 1	-	-	-	-	768	-
Stage 2	-	-	-	-	770	-

Approach	EB	WB	SB			
HCM Control Delay, s	0.1	0	11.7			
HCM LOS			B			

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1279	-	-	-	562	
HCM Lane V/C Ratio	0.003	-	-	-	0.043	
HCM Control Delay (s)	7.8	0	-	-	11.7	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	→		↗	
Traffic Vol, veh/h	11	319	383	15	11	3
Future Vol, veh/h	11	319	383	15	11	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	86	86	58	58
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	336	445	17	19	5
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	462	0	-	0	814	454
Stage 1	-	-	-	-	454	-
Stage 2	-	-	-	-	360	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1099	-	-	-	347	606
Stage 1	-	-	-	-	640	-
Stage 2	-	-	-	-	706	-
Platoon blocked, %	-	-	-	-		
Mov Cap-1 Maneuver	1099	-	-	-	342	606
Mov Cap-2 Maneuver	-	-	-	-	342	-
Stage 1	-	-	-	-	632	-
Stage 2	-	-	-	-	706	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.3	0	15.2			
HCM LOS			C			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1099	-	-	-	377	
HCM Lane V/C Ratio	0.011	-	-	-	0.064	
HCM Control Delay (s)	8.3	0	-	-	15.2	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↗	
Traffic Vol, veh/h	3	251	217	9	11	6
Future Vol, veh/h	3	251	217	9	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	89	89	77	77	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	3	282	282	12	15	8

Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	294	0	-	0	576	288
Stage 1	-	-	-	-	288	-
Stage 2	-	-	-	-	288	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1268	-	-	-	479	751
Stage 1	-	-	-	-	761	-
Stage 2	-	-	-	-	761	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1268	-	-	-	478	751
Mov Cap-2 Maneuver	-	-	-	-	478	-
Stage 1	-	-	-	-	759	-
Stage 2	-	-	-	-	761	-

Approach	EB	WB	SB			
HCM Control Delay, s	0.1	0	11.9			
HCM LOS			B			

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1268	-	-	-	548	
HCM Lane V/C Ratio	0.003	-	-	-	0.044	
HCM Control Delay (s)	7.8	0	-	-	11.9	
HCM Lane LOS	A	A	-	-	B	
HCM 95th %tile Q(veh)	0	-	-	-	0.1	

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑		↗	
Traffic Vol, veh/h	11	332	399	16	11	3
Future Vol, veh/h	11	332	399	16	11	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	86	86	58	58
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	349	464	19	19	5
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	483	0	-	0	847	474
Stage 1	-	-	-	-	474	-
Stage 2	-	-	-	-	373	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1080	-	-	-	332	590
Stage 1	-	-	-	-	626	-
Stage 2	-	-	-	-	696	-
Platoon blocked, %	-	-	-	-		
Mov Cap-1 Maneuver	1080	-	-	-	327	590
Mov Cap-2 Maneuver	-	-	-	-	327	-
Stage 1	-	-	-	-	617	-
Stage 2	-	-	-	-	696	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.3	0	15.7			
HCM LOS			C			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1080	-	-	-	362	
HCM Lane V/C Ratio	0.011	-	-	-	0.067	
HCM Control Delay (s)	8.4	0	-	-	15.7	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0	-	-	-	0.2	